The following excerpt are pages from the North American Product Technical Guide, Volume 2: Anchor Fastening, Edition 22.

Please refer to the publication in its entirety for complete details on this product including data development, product specifications, general suitability, installation, corrosion and spacing and edge distance guidelines.
US&CA: https://submittals.us.hilti.com/PTGVol2/

To consult directly with a team member regarding our anchor fastening products, contact Hilti's team of technical support specialists between the hours of 7:00am – 6:00pm CST.
US: 877-749-6337 or HNATechnicalServices@hilti.com
CA: 1-800-363-4458, ext. 6 or CATechnicalServices@hilti.com

Hilti, Inc.
7250 Dallas Parkway, Suite 1000
Plano, TX 75024

1-800-879-8000
www.hilti.com

3.3.11 KWIK BOLT 3 STAINLESS STEEL AND HOT-DIPPED GALVANIZED EXPANSION ANCHOR

PRODUCT DESCRIPTION

KWIK Bolt 3 stainless steel anchors and hot-dipped galvanized plating

Anchor System		Features and Benefits
 Hot Dipped Galvanized KB3	 Stainless Steel KB3	• Used with Hilti Dust Removal System (DRS) for dustless drilling and installation (compliant with Table 1 of OSHA 1926.1153 regulations for silica dust exposure). • Accurate SafeSet™ installation when using the Hilti SIW-6AT-A22 impact wrench and the SI-AT-A22 Adaptive Torque Module • Length identification code facilitates quality control and inspection after installation. • Through fixture installation and variable thread lengths improve productivity and accommodate various base plate thicknesses. • Raised impact section (Dog Point) prevents thread damage during installation. • Anchor size is same as drill bit size for easy installation. For temporary applications anchors may be driven into drilled holes after usage. • Mechanical expansion allows immediate load application.
 A-18/A22 impact tool		

Uncracked concrete

Hollow drill bit adaptive torque tool (AT)

Profis Anchor design software

Approvals/Listings	
ICC-ES (International Code Council) **2018 International Building Code / International Residential Code (IBC/IRC)**	ESR-2302 in concrete per ACI 318 Ch. 17 / ACI 355.2/ ICC-ES AC193
City of Los Angeles	City of Los Angeles 2020 LABC Supplement (within ESR-2302)
Florida Building Code	2020 FBC with HVHZ
Nuclear Quality Assurance	Qualified under NQA-1 Nuclear Quality Program

MATERIAL SPECIFICATIONS

Carbon steel with hot-dip galvanized plating

Anchor bodies manufactured from carbon steel have the tensile bolt fracture loads shown in table 1.

Carbon steel anchor components have an average zinc plating thickness greater than 43 μm according to ASTM A153, Class C.

Nuts conform to the requirements of ASTM A563, Grade A, Hex.

Washers meet the requirements of ASTM F844.

Stainless steel expansion wedges are manufactured from either AISI Type 304 or Type 316.

Stainless steel

Anchor bodies smaller than 3/4-inch, excluding all KWIK Bolt 3 Countersunk, are produced from AISI Type 304 or Type 316 stainless steel having the bolt fracture loads shown in table 1.

Anchor bodies 3/4-inch and larger, and all stainless steel KWIK Bolt 3 Countersunk anchor bodies, are produced from AISI Type 304 or Type 316 stainless steel having the mechanical properties shown in table 1.

Nuts meet the dimensional requirements of ASTM F594.

Washers meet the dimensional requirements of ANSI B18.22.1, Type A, plain.

Stainless steel expansion wedges for AISI Type 304 are made from either AISI Type 304 or Type 316. Stainless steel expansion wedges for AISI Type 316 anchors are made from type 316. All stainless steel nuts and washers for AISI Type 304 or Type 316 anchors are manufactured from AISI Type 304 or 316, respectively.

Table 1 — Hilti KWIK Bolt 3 Bolt fracture load (lb)[1]

Nominal anchor diameter in.	Hot-dip galvanized	Stainless steel
1/4	no offering	2,900
3/8	no offering	7,200
1/2	12,400	12,400
5/8	19,600	21,900
3/4	28,700	$f_{uta} \geq 76$, $f_{ya} \geq 64$[2]
1	no offering	$f_{uta} \geq 76$, $f_{ya} \geq 64$[2]

[1] Bolt fracture loads are determined by testing in a universal tensile machine for quality control at the manufacturing facility. These loads are not intended for design use. See table 12 for the steel design strengths of stainless steel.

[2] All 3/4-in. stainless steel, and all 1-in. stainless steel material strengths specified by the tensile and yield strengths expressed in (ksi). Bolt fracture loads not applicable for these models.

INSTALLATION PARAMETERS

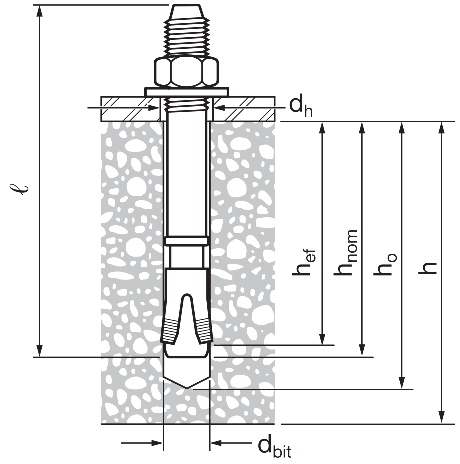


Figure 1 — KWIK Bolt 3 installation

Table 2 — Hilti KWIK Bolt 3 specifications

Setting information	Symbol	Units	Nominal anchor diameter									
			1/4	3/8	1/2		5/8		3/4		1	
Drill bit dia.	d_{bit}	in.	1/4	3/8	1/2		5/8		3/4		1	
Minimum nominal embedment	h_{nom}	in.	1-3/4	2-3/8	2-1/4	3-5/8	3-1/2	4-3/8	4-1/4	5-5/8	4-5/8	6-3/8
		(mm)	(44)	(60)	(57)	(92)	(89)	(111)	(108)	(143)	117	162
Minimum effective embedment	h_{ef}	in.	1-1/2	2	2	3-1/4	3-1/8	4	3-3/4	5	4	5-3/4
		(mm)	(38)	(51)	(51)	(83)	(79)	(102)	(95)	(127)	(102)	(146)
Minimum hole depth	h_o	in.	2	2-5/8	2-5/8	4	3-7/8	4-3/4	4-1/2	5-3/4	5	6-3/4
		(mm)	(51)	(67)	(67)	(102)	(98)	(121)	(114)	(146)	(127)	(171)
Fixture hole dia.	d_h	in.	5/16	7/16	9/16		11/16		13/16		1-1/8	
Anchor length	ℓ		See ordering information									
Installation torque concrete	T_{inst}	ft-lb	4	20	40		60		110		150	
		(Nm)	(5)	(27)	(54)		(81)		(149)		(203)	
Installation torque masonry	T_{inst}	ft-lb	4	15	25		65		120		not recommended	
		(Nm)	(5)	(20)	(34)		(88)		(163)			
Wrench size		in.	7/16	9/16	3/4		15/16		1-1/8		1-1/2	

DESIGN INFORMATION IN CONCRETE PER ACI 318

ACI 318 Chapter 17 design

The load values contained in this section are Hilti Simplified Design Tables. The load tables in this section were developed using the Strength Design parameters and variables of ESR-2302 and the equations within ACI 318 Chapter 17. For a detailed explanation of the Hilti Simplified Design Tables, refer to section 3.1.7. Data tables from ESR-2302 are not contained in this section, but can be found at www.icc-es.org or at www.hilti.com.

Allowable Stress Design or ASD technical information and data tables can be found at www.hilti.com.

Table 3 — Hilti KWIK Bolt 3 stainless steel design strength with concrete / pullout failure in uncracked concrete[1,2,3,4,5]

Nominal anchor diameter in.	Effective embed. in. (mm)	Nominal embed. in. (mm)	Tension - ϕN_n				Shear - ϕV_n			
			f'_c = 2,500 psi lb (kN)	f'_c = 3,000 psi lb (kN)	f'_c = 4,000 psi lb (kN)	f'_c = 6,000 psi lb (kN)	f'_c = 2,500 psi lb (kN)	f'_c = 3,000 psi lb (kN)	f'_c = 4,000 psi lb (kN)	f'_c = 6,000 psi lb (kN)
1/4	1-1/2 (38)	1-3/4 (44)	730 (3.2)	770 (3.4)	840 (3.7)	950 (4.2)	1,545 (6.9)	1,690 (7.5)	1,950 (8.7)	2,390 (10.6)
3/8	2 (51)	2-3/8 (60)	1,925 (8.6)	2,110 (9.4)	2,440 (10.9)	2,985 (13.3)	2,375 (10.6)	2,605 (11.6)	3,005 (13.4)	3,680 (16.4)
1/2	2 (51)	2-1/4 (57)	2,150 (9.6)	2,355 (10.5)	2,720 (12.1)	3,335 (14.8)	2,375 (10.6)	2,605 (11.6)	3,005 (13.4)	3,680 (16.4)
	3-1/4 (83)	3-1/2 (89)	3,920 (17.4)	4,295 (19.1)	4,960 (22.1)	6,070 (27.0)	9,845 (43.8)	10,785 (48.0)	12,450 (55.4)	15,250 (67.8)
5/8	3-1/8 (79)	3-1/2 (89)	4,050 (18.0)	4,435 (19.7)	5,120 (22.8)	6,275 (27.9)	9,280 (41.3)	10,165 (45.2)	11,740 (52.2)	14,380 (64.0)
	4 (102)	4-3/8 (111)	5,090 (22.6)	5,575 (24.8)	6,440 (28.6)	7,885 (35.1)	13,440 (59.8)	14,725 (65.5)	17,000 (75.6)	20,820 (92.6)
3/4	3-3/4 (95)	4-1/4 (108)	5,560 (24.7)	6,090 (27.1)	7,035 (31.3)	8,615 (38.3)	12,200 (54.3)	13,365 (59.5)	15,430 (68.6)	18,900 (84.1)
	5 (127)	5-1/2 (140)	7,040 (31.3)	7,710 (34.3)	8,905 (39.6)	10,905 (48.5)	18,785 (83.6)	20,575 (91.5)	23,760 (105.7)	29,100 (129.4)
1	4 (102)	4-1/2 (114)	6,240 (27.8)	6,835 (30.4)	7,895 (35.1)	9,665 (43.0)	13,440 (59.8)	14,725 (65.5)	17,000 (75.6)	20,820 (92.6)
	5-3/4 (146)	6-1/4 (159)	10,110 (45.0)	11,070 (49.2)	12,785 (56.9)	15,660 (69.7)	23,165 (103.0)	25,375 (112.9)	29,300 (130.3)	35,885 (159.6)

1 See section 3.1.8 to convert design strength value to ASD value.

2 Linear interpolation between embedment depths and concrete compressive strengths is not permitted.

3 Apply spacing, edge distance, and concrete thickness factors in tables 14 to 19 as necessary. Compare to steel values in table 12. The lesser of the values is to be used for the design.

4 Tabular values are for normal-weight concrete only. For lightweight concrete multiply design strength by λ_a as follows: for sand-lightweight, λ_a = 0.68; for all-lightweight, λ_a = 0.60

5 Tabular values are for static loads only. Seismic design is not permitted for uncracked concrete.

Table 4 — Steel design strength for Hilti KWIK Bolt 3 stainless steel anchors[1,2]

Nominal anchor diameter in.	Nominal embedment in. (mm)	Tensile[3] ϕN_{sa} lb (kN)	Shear[4] ϕV_{sa} lb (kN)
1/4	1-3/4 (44)	1,725 (7.7)	1,090 (4.8)
3/8	2-3/8 (60)	5,175 (23.0)	3,235 (14.4)
1/2	2-1/4 (57)	9,490 (42.2)	2,725 (12.1)
	3-1/2 (89)		4,510 (20.1)
5/8	3-1/2 (89)	14,665 (65.2)	5,820 (25.9)
	4-3/8 (111)		9,295 (41.3)
3/4	4-1/4 (108)	16,200 (72.1)	7,735 (34.4)
	5-1/2 (140)		15,305 (68.1)
1	4-1/2 (114)	31,735 (141.2)	8,130 (36.2)
	6-1/4 (159)		17,775 (79.1)

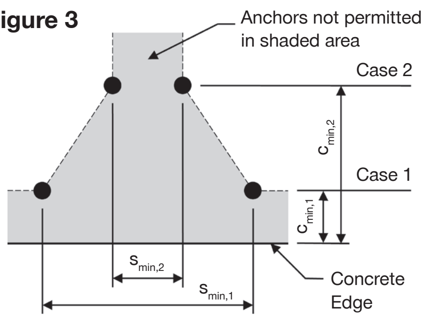
1 See section 3.1.8 to convert design strength value to ASD value.

2 KWIK Bolt 3 stainless steel anchors are to be considered ductile steel elements.

3 Tensile $\phi N_{sa} = \phi A_{se,N} f_{uta}$ as noted in ACI 318 Chapter 17.

4 Shear values determined by static shear tests with $\phi V_{sa} < \phi\ 0.60\ A_{se,V}\ f_{uta}$ as noted in ACI 318 Chapter 17.

Figure 3



For a specific edge distance, the permitted spacing is calculated as follows:

$$s \geq s_{min,2} + \frac{(s_{min,1} - s_{min,2})}{(c_{min,1} - c_{min,2})} (c - c_{min,2})$$

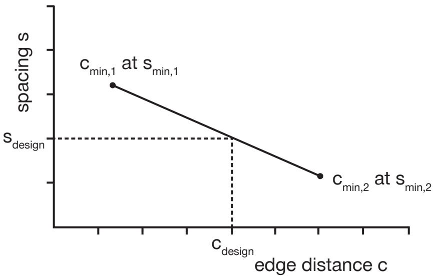


Table 5 — Stainless steel Hilti KWIK Bolt 3 installation parameters[1]

Setting information	Symbol	Units	Nominal anchor diameter d_o														
			1/4	3/8		1/2				5/8			3/4			1	
Effective minimum embedment	h_{ef}	in. (mm)	1-1/2 (38)	2 (51)		2 (51)		3-1/4 (83)		3-1/8 (79)	4 (102)		3-3/4 (95)		5 (127)	4 (102)	5-3/4 (146)
Minimum member thickness	h_{min}	in. (mm)	4 (102)	4 (102)	5 (127)	4 (102)	6 (152)	6 (152)	8 (203)	5 (127)	6 (152)	8 (203)	6 (152)	8 (203)	8 (203)	8 (203)	10 (254)
Case 1	$c_{min,1}$	in. (mm)	1-3/8 (35)	2 (51)	1-5/8 (41)	2-1/2 (68)	1-7/8 (48)	1-5/8 (41)	1-5/8 (41)	3-1/4 (83)	2-1/2 (64)	2-1/2 (64)	3-1/4 (83)	3 (76)	2-7/8 (73)	3-1/2 (89)	3 (76)
	for $s_{min,1}$ ≥	in. (mm)	1-3/4 (44)	4 (102)	3-5/8 (92)	5 (127)	4-5/8 (117)	4-1/2 (114)	4-1/4 (108)	5-5/8 (143)	5-1/4 (133)	5 (127)	7 (178)	6-7/8 (175)	6-5/8 (168)	6-3/4 (172)	6-3/4 (172)
Case 2	$c_{min,2}$	in. (mm)	1-5/8 (41)	3-1/4 (83)	2-1/2 (64)	2-7/8 (73)	2-3/8 (60)	2-3/8 (60)	2-1/8 (54)	3-7/8 (98)	3 (76)	2-3/4 (70)	4-1/8 (105)	3-3/4 (95)	3-3/4 (95)	4-1/4 (108)	3-3/4 (95)
	for $s_{min,2}$ ≥	in. (mm)	1-1/4 (32)	2 (51)	1-3/4 (44)	2-1/2 (64)	2-1/4 (57)	2-1/8 (54)	1-7/8 (48)	3-1/8 (79)	2-1/8 (54)	2-1/8 (54)	4 (102)	3-1/2 (89)	3-1/2 (89)	5 (127)	4-3/4 (121)

1 Linear interpolation is permitted to establish an edge distance and spacing combination between Case 1 and Case 2. Linear interpolation for a specific edge .distance c, where $c_{min,1} < c < c_{min,2}$ will determine the permissible spacings.

Table 6 — Load adjustment factors for 1/4-in. diameter Hilti KWIK Bolt 3 stainless steel anchor in uncracked concrete[1,2]

1/4-in. KB3 stainless steel uncracked uncrete			Spacing factor in tension f_{AN}	Edge distance factor in tension f_{RN}	Spacing factor in shear[3] f_{AV}	Edge distance in shear		Concrete thickness factor in shear[4] f_{HV}
						⊥ toward edge f_{RV}	II to and away from edge f_{RV}	
Embedment h_{nom}		in. (mm)	1-3/4 (44)	1-3/4 (44)	1-3/4 (44)	1-3/4 (44)	1-3/4 (44)	1-3/4 (44)
Spacing (s) / edge distance (c_a) / concrete thickness (h) - in. (mm)	1-1/4	(32)	0.64	n/a	0.56	n/a	n/a	n/a
	1-3/8	(35)	0.65	0.53	0.57	0.26	0.51	n/a
	1-1/2	(38)	0.67	0.56	0.57	0.29	0.56	n/a
	2	(51)	0.72	0.68	0.60	0.45	0.68	n/a
	3	(76)	0.83	1.00	0.65	0.83	1.00	n/a
	3-1/2	(89)	0.89		0.67	1.00		n/a
	4	(102)	0.94		0.70			0.88
	4-1/2	(114)	1.00		0.72			0.94
	5	(127)			0.74			0.99
	5-1/2	(140)			0.77			1.00
	6	(152)			0.79			
	7	(178)			0.84			
	8	(203)			0.89			
	9	(229)			0.94			
	10	(254)			0.99			
	11	(279)			1.00			

1 Linear interpolation not permitted.

2 When combining multiple load adjustment factors (e.g. for a 4 anchor pattern in a corner with thin concrete member) the design can become very conservative. To optimize the design, use PROFIS Engineering or perform anchor calculation using design equations from ACI 318 Chapter 17.

3 Spacing factor reduction in shear, f_{AV}, assumes an influence of a nearby edge. If no edge exists, then $f_{AV} = f_{AN}$.

4 Concrete thickness reduction factor in shear, f_{HV}, assumes an influence of a nearby edge. If no edge exists, then f_{HV} = 1.0.

▭ If a reduction factor value is in a shaded cell, this indicates that this specific edge distance may not be permitted with a certain spacing (or vice versa). Check with table 13 and figure 3 of this section to calculate permissable edge distance, spacing and concrete thickness combinations. Use of Hilti KWIK Bolt 3 anchors with edge distance and spacing dimensions smaller than what is noted in this table is permitted.

Table 7 — Load adjustment factors for 3/8-in. diameter Hilti KWIK Bolt 3 stainless steel anchor in uncracked concrete[1,2]

3/8-in. KB3 stainless steel uncracked concrete			Spacing factor in tension f_{AN}	Edge distance factor in tension f_{RN}	Spacing factor in shear[3] f_{AV}	Edge distance in shear ⊥ toward edge f_{RV}	Edge distance in shear II to and away from edge f_{RV}	Concrete thickness factor in shear[4] f_{HV}
Embedment h_{nom}	in.	(mm)	2-3/8 (60)	2-3/8 (60)	2-3/8 (60)	2-3/8 (60)	2-3/8 (60)	2-3/8 (60)
Spacing (s) / edge distance (c_a) / concrete thickness (h) - in. (mm)	2	(51)	0.67	0.51	0.58	0.35	0.51	n/a
	2-1/2	(64)	0.71	0.60	0.60	0.49	0.60	n/a
	3	(76)	0.75	0.69	0.62	0.64	0.69	n/a
	3-1/2	(89)	0.79	0.80	0.64	0.81	0.81	n/a
	4	(102)	0.83	0.91	0.67	0.99	0.99	0.81
	4-1/2	(114)	0.88	1.00	0.69	1.00	1.00	0.86
	5	(127)	0.92		0.71			0.91
	6	(152)	1.00		0.75			1.00
	7	(178)			0.79			
	8	(203)			0.83			
	9	(229)			0.87			
	10	(254)			0.91			
	11	(279)			0.95			
	12	(305)			1.00			
	14	(356)						

Table 8 — Load adjustment factors for 1/2-in. diameter Hilti KWIK Bolt 3 stainless steel anchor in uncracked concrete[1,2]

1/2-in. KB3 stainless steel uncracked concrete			Spacing factor in tension f_{AN}		Edge distance factor in tension f_{RN}		Spacing factor in shear[3] f_{AV}		Edge distance in shear ⊥ toward edge f_{RV}		Edge distance in shear II to and away from edge f_{RV}		Concrete thickness factor in shear[4] f_{HV}	
Embedment h_{nom}	in.	(mm)	2-1/4 (57)	3-1/2 (89)	2-1/4 (57)	3-1/2 (89)	2-1/4 (57)	3-1/2 (89)	2-1/4 (57)	3-1/2 (89)	2-1/4 (57)	3-1/2 (89)	2-1/4 (57)	3-1/2 (89)
Spacing (s) / edge distance (c_a) / concrete thickness (h) - in. (mm)	1-5/8	(41)	n/a	n/a	n/a	0.39	n/a	n/a	n/a	0.07	n/a	0.15	n/a	n/a
	2	(51)	n/a	n/a	n/a	0.42	n/a	n/a	n/a	0.10	n/a	0.20	n/a	n/a
	2-1/8	(54)	n/a	0.61	n/a	0.43	n/a	0.54	n/a	0.11	n/a	0.22	n/a	n/a
	2-1/2	(64)	0.71	0.63	0.54	0.47	0.61	0.55	0.53	0.14	0.54	0.28	n/a	n/a
	3	(76)	0.75	0.65	0.62	0.52	0.63	0.55	0.70	0.19	0.70	0.37	n/a	n/a
	3-1/2	(89)	0.79	0.68	0.72	0.57	0.65	0.56	0.88	0.23	0.88	0.47	n/a	n/a
	4	(102)	0.83	0.71	0.82	0.62	0.68	0.57	1.00	0.29	1.00	0.57	0.84	n/a
	4-1/2	(114)	0.88	0.73	0.92	0.68	0.70	0.58		0.34		0.68	0.89	n/a
	5	(127)	0.92	0.76	1.00	0.74	0.72	0.59		0.40		0.74	0.94	n/a
	6	(152)	1.00	0.81		0.89	0.76	0.61		0.53		0.89	1.00	0.66
	7	(178)		0.86		1.00	0.81	0.63		0.66		1.00		0.71
	8	(203)		0.91			0.85	0.64		0.81				0.76
	9	(229)		0.96			0.89	0.66		0.97				0.81
	10	(254)		1.00			0.94	0.68		1.00				0.85
	11	(279)					0.98	0.70						0.89
	12	(305)					1.00	0.72						0.93
	14	(356)						0.75						1.00
	16	(406)						0.79						
	18	(457)						0.83						
	20	(508)						0.86						
	> 24	(610)						0.93						

1 Linear interpolation not permitted.

2 When combining multiple load adjustment factors (e.g. for a 4 anchor pattern in a corner with thin concrete member) the design can become very conservative. To optimize the design, use PROFIS Engineering or perform anchor calculation using design equations from ACI 318 Chapter 17.

3 Spacing factor reduction in shear, f_{AV}, assumes an influence of a nearby edge. If no edge exists, then $f_{AV} = f_{AN}$.

4 Concrete thickness reduction factor in shear, f_{HV}, assumes an influence of a nearby edge. If no edge exists, then f_{HV} = 1.0.

▨ If a reduction factor value is in a shaded cell, this indicates that this specific edge distance may not be permitted with a certain spacing (or vice versa). Check with table 13 and figure 3 of this section to calculate permissable edge distance, spacing and concrete thickness combinations. Use of Hilti KWIK Bolt 3 anchors with edge distance and spacing dimensions smaller than what is noted in this table is permitted.

Table 9 — Load adjustment factors for 5/8-in. diameter Hilti KWIK Bolt 3 stainless steel anchor in uncracked concrete[1,2]

5/8-in. KB3 stainless steel uncracked concrete			Spacing factor in tension f_{AN}		Edge distance factor in tension f_{RN}		Spacing factor in shear[3] f_{AV}		Edge distance in shear				Concrete thickness factor in shear[4] f_{HV}	
									⊥ toward edge f_{RV}		II to and away from edge f_{RV}			
Embedment h_{nom}		in. (mm)	3-1/2 (89)	4-3/8 (111)	3-1/2 (89)	4-3/8 (111)	3-1/2 (89)	4-3/8 (111)	3-1/2 (89)	4-3/8 (111)	3-1/2 (89)	4-3/8 (111)	3-1/2 (89)	4-3/8 (111)
Spacing (s) / edge distance (c_a) / concrete thickness (h) - in. (mm)	2-1/8	(54)	n/a	0.59	n/a	n/a	n/a	0.53	n/a	n/a	n/a	n/a	n/a	n/a
	2-1/2	(64)	n/a	0.60	n/a	0.37	n/a	0.54	n/a	0.12	n/a	0.23	n/a	n/a
	3	(76)	n/a	0.63	n/a	0.40	n/a	0.55	n/a	0.15	n/a	0.30	n/a	n/a
	3-1/8	(79)	0.67	0.63	n/a	0.41	0.56	0.55	n/a	0.16	n/a	0.32	n/a	n/a
	3-1/4	(83)	0.67	0.64	0.49	0.42	0.56	0.55	0.24	0.17	0.47	0.34	n/a	n/a
	3-1/2	(89)	0.69	0.65	0.51	0.44	0.57	0.56	0.26	0.19	0.51	0.38	n/a	n/a
	4	(102)	0.71	0.67	0.56	0.47	0.58	0.56	0.32	0.23	0.56	0.47	n/a	n/a
	5	(127)	0.77	0.71	0.68	0.55	0.60	0.58	0.45	0.33	0.68	0.55	0.63	n/a
	6	(152)	0.82	0.75	0.81	0.63	0.62	0.59	0.59	0.43	0.81	0.63	0.69	0.62
	7	(178)	0.87	0.79	0.95	0.74	0.64	0.61	0.75	0.54	0.95	0.74	0.74	0.67
	8	(203)	0.93	0.83	1.00	0.84	0.66	0.63	0.91	0.66	1.00	0.84	0.79	0.71
	9	(229)	0.98	0.88		0.95	0.68	0.64	1.00	0.79		0.95	0.84	0.75
	10	(254)	1.00	0.92		1.00	0.70	0.66		0.92		1.00	0.89	0.80
	11	(279)		0.96			0.72	0.67		1.00			0.93	0.83
	12	(305)		1.00			0.74	0.69					0.97	0.87
	14	(356)					0.77	0.72					1.00	0.94
	16	(406)					0.81	0.75						1.00
	18	(457)					0.85	0.78						
	20	(508)					0.89	0.82						
	24	(610)					0.97	0.88						
	> 30	(762)					1.00	0.97						

Table 10 — Load adjustment factors for 3/4-in. diameter Hilti KWIK Bolt 3 stainless steel anchor in uncracked concrete[1,2]

3/4-in. KB3 stainless steel uncracked concrete			Spacing factor in tension f_{AN}		Edge distance factor in tension f_{RN}		Spacing factor in shear[3] f_{AV}		Edge distance in shear				Concrete thickness factor in shear[4] f_{HV}	
									⊥ toward edge f_{RV}		II to and away from edge f_{RV}			
Embedment h_{nom}		in. (mm)	4-1/4 (108)	5-1/2 (140)	4-1/4 (108)	5-1/2 (140)	4-1/4 (108)	5-1/2 (140)	4-1/4 (108)	5-1/2 (140)	4-1/4 (108)	5-1/2 (140)	4-1/4 (108)	5-1/2 (140)
Spacing (s) / edge distance (c_a) / concrete thickness (h) - in. (mm)	2-7/8	(73)	n/a	n/a	n/a	0.43	n/a	n/a	n/a	0.11	n/a	0.23	n/a	n/a
	3	(76)	n/a	n/a	n/a	0.44	n/a	n/a	n/a	0.12	n/a	0.24	n/a	n/a
	3-1/4	(83)	n/a	n/a	0.37	0.46	n/a	n/a	0.20	0.14	0.37	0.27	n/a	n/a
	3-1/2	(89)	n/a	0.62	0.39	0.47	n/a	0.55	0.22	0.15	0.39	0.30	n/a	n/a
	4	(102)	0.68	0.63	0.42	0.51	0.57	0.55	0.27	0.18	0.42	0.37	n/a	n/a
	4-1/2	(114)	0.70	0.65	0.45	0.54	0.58	0.56	0.32	0.22	0.45	0.44	n/a	n/a
	5	(127)	0.72	0.67	0.49	0.58	0.59	0.57	0.38	0.26	0.49	0.52	n/a	n/a
	6	(152)	0.77	0.70	0.57	0.65	0.60	0.58	0.49	0.34	0.57	0.65	0.65	n/a
	7	(178)	0.81	0.73	0.67	0.73	0.62	0.59	0.62	0.43	0.67	0.73	0.70	n/a
	8	(203)	0.86	0.77	0.76	0.82	0.64	0.61	0.76	0.52	0.76	0.82	0.75	0.66
	9	(229)	0.90	0.80	0.86	0.92	0.66	0.62	0.91	0.62	0.91	0.92	0.79	0.70
	10	(254)	0.94	0.83	0.95	1.00	0.67	0.64	1.00	0.73	1.00	1.00	0.83	0.74
	11	(279)	0.99	0.87	1.00		0.69	0.65		0.84			0.87	0.77
	12	(305)	1.00	0.90			0.71	0.66		0.96			0.91	0.81
	14	(356)		0.97			0.74	0.69		1.00			0.99	0.87
	16	(406)		1.00			0.78	0.72					1.00	0.93
	18	(457)					0.81	0.74						0.99
	20	(508)					0.85	0.77						1.00
	24	(610)					0.92	0.82						
	30	(762)					1.00	0.91						
	> 36	(914)						0.99						

1 Linear interpolation not permitted.

2 When combining multiple load adjustment factors (e.g. for a 4 anchor pattern in a corner with thin concrete member) the design can become very conservative. To optimize the design, use PROFIS Engineering or perform anchor calculation using design equations from ACI 318 Chapter 17.

3 Spacing factor reduction in shear, f_{AV}, assumes an influence of a nearby edge. If no edge exists, then $f_{AV} = f_{AN}$.

4 Concrete thickness reduction factor in shear, f_{HV}, assumes an influence of a nearby edge. If no edge exists, then f_{HV} = 1.0.

▨ If a reduction factor value is in a shaded cell, this indicates that this specific edge distance may not be permitted with a certain spacing (or vice versa). Check with table 13 and figure 3 of this section to calculate permissable edge distance, spacing and concrete thickness combinations. Use of Hilti KWIK Bolt 3 anchors with edge distance and spacing dimensions smaller than what is noted in this table is permitted.

Table 11 — Load adjustment factors for 1-in. diameter Hilti KWIK Bolt 3 stainless steel anchor in uncracked concrete[1,2]

1-in. KB3 stainless steel uncracked concrete			Spacing factor in tension f_{AN}		Edge distance factor in tension f_{RN}		Spacing factor in shear[3] f_{AV}		Edge distance in shear				Concrete thickness factor in shear[4] f_{HV}	
									⊥ toward edge f_{RV}		II to and away from edge f_{RV}			
Embedment h_{nom}		in. (mm)	4-1/2 (114)	6-1/4 (159)	4-1/2 (114)	6-1/4 (159)	4-1/2 (114)	6-1/4 (159)	4-1/2 (114)	6-1/4 (159)	4-1/2 (114)	6-1/4 (159)	4-1/2 (114)	6-1/4 (159)
Spacing (s) / edge distance (c_a) / concrete thickness (h) - in. (mm)	3	(76)	n/a	n/a	n/a	0.43	n/a	n/a	n/a	0.10	n/a	0.20	n/a	n/a
	3-1/2	(89)	n/a	n/a	0.42	0.45	n/a	n/a	0.21	0.12	0.42	0.25	n/a	n/a
	4	(102)	n/a	n/a	0.45	0.48	n/a	n/a	0.26	0.15	0.45	0.30	n/a	n/a
	4-1/2	(114)	n/a	n/a	0.49	0.51	n/a	n/a	0.31	0.18	0.49	0.36	n/a	n/a
	4-3/4	(121)	n/a	0.64	0.50	0.53	n/a	0.56	0.34	0.20	0.50	0.39	n/a	n/a
	5	(127)	0.71	0.64	0.52	0.54	0.59	0.56	0.37	0.21	0.52	0.43	n/a	n/a
	6	(152)	0.75	0.67	0.60	0.60	0.60	0.57	0.48	0.28	0.60	0.56	n/a	n/a
	7	(178)	0.79	0.70	0.70	0.67	0.62	0.58	0.61	0.35	0.70	0.67	n/a	n/a
	8	(203)	0.83	0.73	0.80	0.74	0.64	0.60	0.74	0.43	0.80	0.74	0.74	n/a
	9	(229)	0.88	0.76	0.90	0.82	0.65	0.61	0.89	0.51	0.90	0.82	0.78	n/a
	10	(254)	0.92	0.79	1.00	0.91	0.67	0.62	1.00	0.60	1.00	0.91	0.83	0.69
	11	(279)	0.96	0.82		1.00	0.69	0.63		0.69		1.00	0.87	0.72
	12	(305)	1.00	0.85			0.70	0.64		0.79			0.91	0.76
	14	(356)		0.91			0.74	0.67		1.00			0.98	0.82
	16	(406)		0.96			0.77	0.69					1.00	0.87
	18	(457)		1.00			0.81	0.71						0.92
	20	(508)					0.84	0.74						0.98
	24	(610)					0.91	0.79						1.00
	30	(762)					1.00	0.86						
	> 36	(914)						0.93						

1 Linear interpolation not permitted.

2 When combining multiple load adjustment factors (e.g. for a 4 anchor pattern in a corner with thin concrete member) the design can become very conservative. To optimize the design, use PROFIS Engineering or perform anchor calculation using design equations from ACI 318 Chapter 17.

3 Spacing factor reduction in shear, f_{AV}, assumes an influence of a nearby edge. If no edge exists, then $f_{AV} = f_{AN}$.

4 Concrete thickness reduction factor in shear, f_{HV}, assumes an influence of a nearby edge. If no edge exists, then f_{HV} = 1.0.

▨ If a reduction factor value is in a shaded cell, this indicates that this specific edge distance may not be permitted with a certain spacing (or vice versa). Check with table 13 and figure 3 of this section to calculate permissable edge distance, spacing and concrete thickness combinations. Use of Hilti KWIK Bolt 3 anchors with edge distance and spacing dimensions smaller than what is noted in this table is permitted.

Table 12 — Hilti KWIK Bolt 3 hot-dip galvanized design strength with concrete/pullout failure in uncracked concrete[1,2,3,4,5]

Nominal anchor diameter in.	Effective embed. in. (mm)	Nominal embed. in. (mm)	Tension - ϕN_n				Shear - ϕV_n			
			f'_c = 2,500 psi lb (kN)	f'_c = 3,000 psi lb (kN)	f'_c = 4,000 psi lb (kN)	f'_c = 6,000 psi lb (kN)	f'_c = 2,500 psi lb (kN)	f'_c = 3,000 psi lb (kN)	f'_c = 4,000 psi lb (kN)	f'_c = 6,000 psi lb (kN)
1/2	2 (51)	2-1/4 (57)	2,205 (9.8)	2,415 (10.7)	2,790 (12.4)	3,420 (15.2)	2,375 (10.6)	2,605 (11.6)	3,005 (13.4)	3,680 (16.4)
	3-1/4 (83)	3-1/2 (89)	4,250 (18.9)	4,655 (20.7)	5,375 (23.9)	6,585 (29.3)	9,845 (43.8)	10,785 (48.0)	12,450 (55.4)	15,250 (67.8)
5/8	3-1/8 (79)	3-1/2 (89)	4,200 (18.7)	4,605 (20.5)	5,315 (23.6)	6,510 (29.0)	9,280 (41.3)	10,165 (45.2)	11,740 (52.2)	14,380 (64.0)
	4 (102)	4-3/8 (111)	5,860 (26.1)	6,420 (28.6)	7,415 (33.0)	9,080 (40.4)	13,440 (59.8)	14,725 (65.5)	17,000 (75.6)	20,820 (92.6)
3/4	3-3/4 (95)	4-1/4 (108)	5,665 (25.2)	6,205 (27.6)	7,165 (31.9)	8,775 (39.0)	12,200 (54.3)	13,365 (59.5)	15,430 (68.6)	18,900 (84.1)
	5 (127)	5-1/2 (140)	6,615 (29.4)	7,245 (32.2)	8,365 (37.2)	10,245 (45.6)	18,785 (83.6)	20,575 (91.5)	23,760 (105.7)	29,100 (129.4)

1 See section 3.1.8 to convert design strength value to ASD value.

2 Linear interpolation between embedment depths and concrete compressive strengths is not permitted.

3 Apply spacing, edge distance, and concrete thickness factors in tables 23 to 25 as necessary. Compare to steel values in table 21. The lesser of the values is to be used for the design.

4 Tabular values are for normal-weight concrete only. For lightweight concrete multiply design strength by λ_a as follows: for sand-lightweight, λ = 0.68; for all-lightweight, λ = 0.60

5 Tabular values are for static loads only. Seismic design is not permitted for uncracked concrete.

Table 13 — Steel design strength for Hilti KWIK Bolt 3 hot-dip galvanized anchors[1,2]

Nominal anchor diameter in.	Nominal embedment in. (mm)	Tensile ϕN_{sa}[3] lb (kN)	Shear ϕV_{sa}[4] lb (kN)
1/2	2-1/4 (57)	8,745 (38.9)	2,925 (13.0)
	3-1/2 (89)		3,815 (17.0)
5/8	3-1/2 (89)	13,515 (60.1)	7,565 (33.7)
	4-3/8 (111)		
3/4	4-1/4 (108)	19,080 (84.9)	11,050 (49.2)
	5-1/2 (140)		

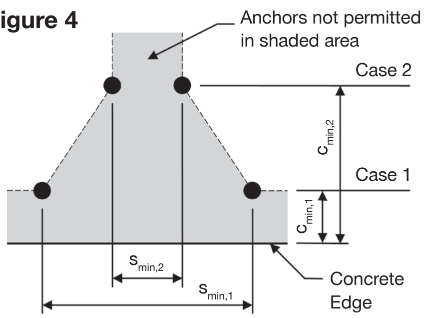
1 See section 3.1.8 to convert design strength value to ASD value.

2 KWIK Bolt 3 carbon steel anchors are to be considered ductile steel elements.

3 Tensile $\phi N_{sa} = \phi A_{se,N} f_{uta}$ as noted in ACI 318 Chapter 17.

4 Shear values determined by static shear tests with $\phi V_{sa} < \phi\ 0.60\ A_{se,V}\ f_{uta}$ as noted in ACI 318 Chapter 17.

Figure 4



For a specific edge distance, the permitted spacing is calculated as follows:

$$s \geq s_{min,2} + \frac{(s_{min,1} - s_{min,2})}{(c_{min,1} - c_{min,2})}\ (c - c_{min,2})$$

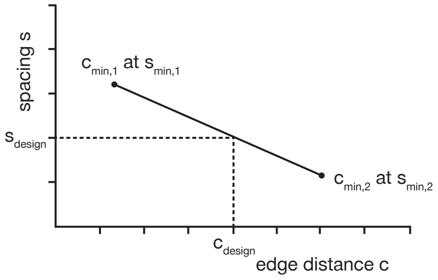


Table 14 — Hot-dip galvanized KWIK Bolt 3 installation parameters[1]

			Nominal anchor diameter d_o										
Setting information	Symbol	Units	1/2				5/8			3/4			
Effective minimum embedment	h_{ef}	in. (mm)	2 (51)		3-1/4 (83)		3-1/8 (79)	4 (102)		3-3/4 (95)		5 (127)
Minimum member thickness	h_{min}	in. (mm)	4 (102)	6 (152)	6 (152)	8 (203)	5 (127)	6 (152)	8 (203)	6 (152)	8 (203)	8 (203)
Case 1	$c_{min,1}$	in. (mm)	3-1/4 (83)	2-5/8 (67)	2 (51)		2-1/4 (57)	2 (51)	1-78 (48)	3-1/2 (89)		3-5/8 (92)
	for $s_{min,1}$ ≥	in. (mm)	6-1/4 (158)	5-1/2 (140)	4-7/8 (124)		5-1/4 (133)	5 (127)	4-3/4 (121)	7-1/2 (191)		7-3/8 (187)
Case 2	$c_{min,2}$	in. (mm)	3-3/4 (95)	2-3/4 (70)	2-5/8 (67)	2-1/4 (57)	3-1/2 (89)	2-1/2 (64)	2-1/4 (57)	6-1/2 (165)		4-3/4 (121)
	for $s_{min,2}$ ≥	in. (mm)	3-1/8 (79)	2-3/4 (70)	2-3/8 (60)	2-1/8 (54)	2-1/2 (64)	2-1/8 (54)	2-1/8 (54)	4 (102)		3-7/8 (98)

1 Linear interpolation is permitted to establish an edge distance and spacing combination between Case 1 and Case 2. Linear interpolation for a specific edge .distance c, where $c_{min,1} < c < c_{min,2}$ will determine the permissible spacings.

Table 15 — Load adjustment factors for 1/2-in. diameter Hilti KWIK Bolt 3 hot-dip galvanized anchor in uncracked concrete[1,2]

1/2-in. KB3 hot-dip galvanized uncracked concrete			spacing factor in tension f_{AN}		edge distance factor in tension f_{RN}		spacing factor in shear[3] f_{AV}		Edge distance in shear				Conc. thickness factor in shear[4] f_{HV}	
									⊥ toward edge f_{RV}		II to and away from edge f_{RV}			
Embedment h_{nom}		in. (mm)	2-1/4 (57)	3-1/2 (89)	2-1/4 (57)	3-1/2 (89)	2-1/4 (57)	3-1/2 (89)	2-1/4 (57)	3-1/2 (89)	2-1/4 (57)	3-1/2 (89)	2-1/4 (57)	3-1/2 (89)
Spacing (s) / edge distance (c_a) / concrete thickness (h) - in. (mm)	2	(51)	n/a	n/a	n/a	0.38	n/a	n/a	n/a	0.10	n/a	0.20	n/a	n/a
	2-3/8	(60)	n/a	0.62	n/a	0.41	n/a	0.54	n/a	0.13	n/a	0.26	n/a	n/a
	2-1/2	(64)	n/a	0.63	n/a	0.42	n/a	0.55	n/a	0.14	n/a	0.28	n/a	n/a
	3	(76)	n/a	0.65	n/a	0.46	n/a	0.55	n/a	0.19	n/a	0.37	n/a	n/a
	3-1/8	(79)	0.76	0.66	n/a	0.48	0.64	0.56	n/a	0.20	n/a	0.40	n/a	n/a
	3-1/4	(83)	0.77	0.67	0.67	0.49	0.64	0.56	0.79	0.21	0.79	0.42	n/a	n/a
	3-1/2	(89)	0.79	0.68	0.72	0.51	0.65	0.56	0.88	0.23	0.88	0.47	n/a	n/a
	4	(102)	0.83	0.71	0.82	0.56	0.68	0.57	1.00	0.29	1.00	0.56	0.84	n/a
	4-1/2	(114)	0.88	0.73	0.92	0.61	0.70	0.58		0.34		0.61	0.89	n/a
	5	(127)	0.92	0.76	1.00	0.67	0.72	0.59		0.40		0.67	0.94	n/a
	6	(152)	1.00	0.81		0.80	0.76	0.61		0.53		0.80	1.00	0.66
	7	(178)	1.00	0.86		0.93	0.81	0.63		0.66		0.93		0.71
	8	(203)		0.91		1.00	0.85	0.64		0.81		1.00		0.76
	9	(229)		0.96			0.89	0.66		0.97				0.81
	10	(254)		1.00			0.94	0.68		1.00				0.85
	11	(279)					0.98	0.70						0.89
	12	(305)					1.00	0.72						0.93
	14	(356)						0.75						1.00
	16	(406)						0.79						
	18	(457)						0.83						
	20	(508)						0.86						
	> 24	(610)						0.93						

1 Linear interpolation not permitted.

2 When combining multiple load adjustment factors (e.g. for a 4 anchor pattern in a corner with thin concrete member) the design can become very conservative. To optimize the design, use PROFIS Engineering or perform anchor calculation using design equations from ACI 318 Chapter 17.

3 Spacing factor reduction in shear, f_{AV}, assumes an influence of a nearby edge. If no edge exists, then $f_{AV} = f_{AN}$.

4 Concrete thickness reduction factor in shear, f_{HV}, assumes an influence of a nearby edge. If no edge exists, then f_{HV} = 1.0.

▨ If a reduction factor value is in a shaded cell, this indicates that this specific edge distance may not be permitted with a certain spacing (or vice versa). Check with table 22 and figure 4 of this section to calculate permissable edge distance, spacing and concrete thickness combinations. Use of Hilti KWIK Bolt 3 anchors with edge distance and spacing dimensions smaller than what is noted in this table is permitted.

Table 16 — Load adjustment factors for 5/8-in. diameter Hilti KWIK Bolt 3 hot-dip galvanized anchor in uncracked concrete^{1,2}

5/8-in. KB3 hot-dip galvanized uncracked concrete			Spacing factor in tension f_{AN}		Edge distance factor in tension f_{RN}		Spacing factor in shear^{3} f_{AV}		Edge distance in shear				Conc. thickness factor in shear^{4} f_{HV}	
									⊥ toward edge f_{RV}		II to and away from edge f_{RV}			
Embedment h_{nom}	in.	(mm)	3-1/2 (89)	4-3/8 (111)	3-1/2 (89)	4-3/8 (111)	3-1/2 (89)	4-3/8 (111)	3-1/2 (89)	4-3/8 (111)	3-1/2 (89)	4-3/8 (111)	3-1/2 (89)	4-3/8 (111)
Spacing (s) / edge distance (c_a) / concrete thickness (h) - in. (mm)	2	(51)	n/a	n/a	n/a	0.34	n/a	n/a	n/a	0.08	n/a	0.17	n/a	n/a
	2-1/8	(54)	n/a	0.59	n/a	0.34	n/a	0.53	n/a	0.09	n/a	0.18	n/a	n/a
	2-1/4	(57)	n/a	0.59	0.38	0.35	n/a	0.54	0.14	0.10	0.27	0.20	n/a	n/a
	2-1/2	(64)	0.63	0.60	0.41	0.37	0.55	0.54	0.16	0.12	0.32	0.23	n/a	n/a
	3	(76)	0.66	0.63	0.45	0.40	0.56	0.55	0.21	0.15	0.42	0.30	n/a	n/a
	3-1/2	(89)	0.69	0.65	0.50	0.44	0.57	0.56	0.26	0.19	0.50	0.38	n/a	n/a
	4	(102)	0.71	0.67	0.54	0.47	0.58	0.56	0.32	0.23	0.54	0.47	n/a	n/a
	4-1/2	(114)	0.74	0.69	0.60	0.51	0.59	0.57	0.38	0.28	0.60	0.51	n/a	n/a
	5	(127)	0.77	0.71	0.66	0.55	0.60	0.58	0.45	0.33	0.66	0.55	0.63	n/a
	6	(152)	0.82	0.75	0.79	0.63	0.62	0.59	0.59	0.43	0.79	0.63	0.69	0.62
	7	(178)	0.87	0.79	0.92	0.74	0.64	0.61	0.75	0.54	0.92	0.74	0.74	0.67
	8	(203)	0.93	0.83	1.00	0.84	0.66	0.63	0.91	0.66	1.00	0.84	0.79	0.71
	9	(229)	0.98	0.88		0.95	0.68	0.64	1.00	0.79		0.95	0.84	0.75
	10	(254)	1.00	0.92		1.00	0.70	0.66		0.92		1.00	0.89	0.80
	11	(279)		0.96			0.72	0.67		1.00			0.93	0.83
	12	(305)		1.00			0.74	0.69					0.97	0.87
	14	(356)					0.77	0.72					1.00	0.94
	16	(406)					0.81	0.75						1.00
	18	(457)					0.85	0.78						
	20	(508)					0.89	0.82						
	24	(610)					0.97	0.88						
	> 30	(762)					1.00	0.97						

Table 17 — Load adjustment factors for 3/4-in. diameter Hilti KWIK Bolt 3 hot-dip galvanized anchor in uncracked concrete^{1,2}

3/4-in. KB3 hot-dip galvanized uncracked concrete			Spacing factor in tension f_{AN}		Edge distance factor in tension f_{RN}		Spacing factor in shear^{3} f_{AV}		Edge distance in shear				Conc. thickness factor in shear^{4} f_{HV}	
									⊥ toward edge f_{RV}		II to and away from edge f_{RV}			
Embedment h_{nom}	in.	(mm)	4-1/4 (108)	5-1/2 (140)	4-1/4 (108)	5-1/2 (140)	4-1/4 (108)	5-1/2 (140)	4-1/4 (108)	5-1/2 (140)	4-1/4 (108)	5-1/2 (140)	4-1/4 (108)	5-1/2 (140)
Spacing (s) / edge distance (c_a) / concrete thickness (h) - in. (mm)	3-1/2	(89)	n/a	n/a	0.41	n/a	n/a	n/a	0.22	n/a	0.41	n/a	n/a	n/a
	3-5/8	(92)	n/a	n/a	0.42	0.49	n/a	n/a	0.23	0.16	0.42	0.32	n/a	n/a
	3-7/8	(98)	n/a	0.63	0.44	0.51	n/a	0.55	0.26	0.18	0.44	0.35	n/a	n/a
	4	(102)	0.68	0.63	0.45	0.52	0.57	0.55	0.27	0.18	0.45	0.37	n/a	n/a
	4-1/2	(114)	0.70	0.65	0.49	0.56	0.58	0.56	0.32	0.22	0.49	0.44	n/a	n/a
	5	(127)	0.72	0.67	0.53	0.59	0.59	0.57	0.38	0.26	0.53	0.52	n/a	n/a
	5-1/2	(140)	0.74	0.68	0.57	0.63	0.60	0.57	0.43	0.30	0.57	0.60	n/a	n/a
	6	(152)	0.77	0.70	0.62	0.67	0.60	0.58	0.49	0.34	0.62	0.67	0.65	n/a
	7	(178)	0.81	0.73	0.72	0.75	0.62	0.59	0.62	0.43	0.72	0.75	0.70	n/a
	8	(203)	0.86	0.77	0.82	0.84	0.64	0.61	0.76	0.52	0.82	0.84	0.75	0.66
	9	(229)	0.90	0.80	0.92	0.95	0.66	0.62	0.91	0.62	0.92	0.95	0.79	0.70
	10	(254)	0.94	0.83	1.00	1.00	0.67	0.64	1.00	0.73	1.00	1.00	0.83	0.74
	11	(279)	0.99	0.87			0.69	0.65		0.84			0.87	0.77
	12	(305)	1.00	0.90			0.71	0.66		0.96			0.91	0.81
	14	(356)		0.97			0.74	0.69		1.00			0.99	0.87
	16	(406)		1.00			0.78	0.72					1.00	0.93
	18	(457)					0.81	0.74						0.99
	20	(508)					0.85	0.77						1.00
	24	(610)					0.92	0.82						
	30	(762)					1.00	0.91						
	> 36	(914)						0.99						

1 Linear interpolation not permitted.

2 When combining multiple load adjustment factors (e.g. for a 4 anchor pattern in a corner with thin concrete member) the design can become very conservative. To optimize the design, use PROFIS Engineering or perform anchor calculation using design equations from ACI 318 Chapter 17.

3 Spacing factor reduction in shear, f_{AV}, assumes an influence of a nearby edge. If no edge exists, then $f_{AV} = f_{AN}$.

4 Concrete thickness reduction factor in shear, f_{HV}, assumes an influence of a nearby edge. If no edge exists, then f_{HV} = 1.0.

▨ If a reduction factor value is in a shaded cell, this indicates that this specific edge distance may not be permitted with a certain spacing (or vice versa). Check with table 22 and figure 4 of this section to calculate permissable edge distance, spacing and concrete thickness combinations. Use of Hilti KWIK Bolt 3 anchors with edge distance and spacing dimensions smaller than what is noted in this table is permitted.

Table 18 — Hilti KWIK Bolt 3 stainless steel design strength in the soffit of uncracked lightweight concrete over metal deck[1,2,3,4,5,7,8]

Nominal anchor diameter in.	Effective embed. in. (mm)	Nominal embed. in. (mm)	Loads according to figure 5			
			Tension - ϕN_n		Shear - ϕV_n	
			f'_c = 3,000 psi lb (kN)	f'_c = 4,000 psi lb (kN)	f'_c = 3,000 psi lb (kN)	f'_c = 4,000 psi lb (kN)
1/4	1-1/2 (38)	1-3/4 (44)	1,175 (5.2)	1,355 (6.0)	1,315 (5.8)	1,315 (5.8)
3/8	2 (51)	2-3/8 (60)	1,675 (7.5)	1,935 (8.6)	1,675 (7.5)	1,675 (7.5)
1/2	2 (51)	2-1/4 (57)	1,265 (5.6)	1,460 (6.5)	1,135 (5.0)	1,135 (5.0)
	3-1/4 (83)	3-1/2 (89)				
5/8	3-1/8 (79)	3-1/2 (89)	2,880 (12.8)	3,325 (14.8)	3,700 (16.5)	3,700 (16.5)
	4 (102)	4-3/8 (111)				

1 See section 3.1.8 to convert design strength value to ASD value.

2 Linear interpolation between embedment depths and concrete compressive strengths is not permitted.

3 Tabular value is for one anchor per flute. Minimum spacing along the length of the flute is 3 x h_{ef} (effective embedment).

4 Tabular values are lightweight concrete and no additional reduction factor is needed.

5 No additional reduction factors for spacing or edge distance need to be applied.

6 Comparison to steel values in table 4 is not required. Values in tables 26 control.

7 Comparison to steel values in table 12 is not required. Values in tables 27 control.

8 Tabular values are for static loads only. Seismic design is not permitted for uncracked concrete.

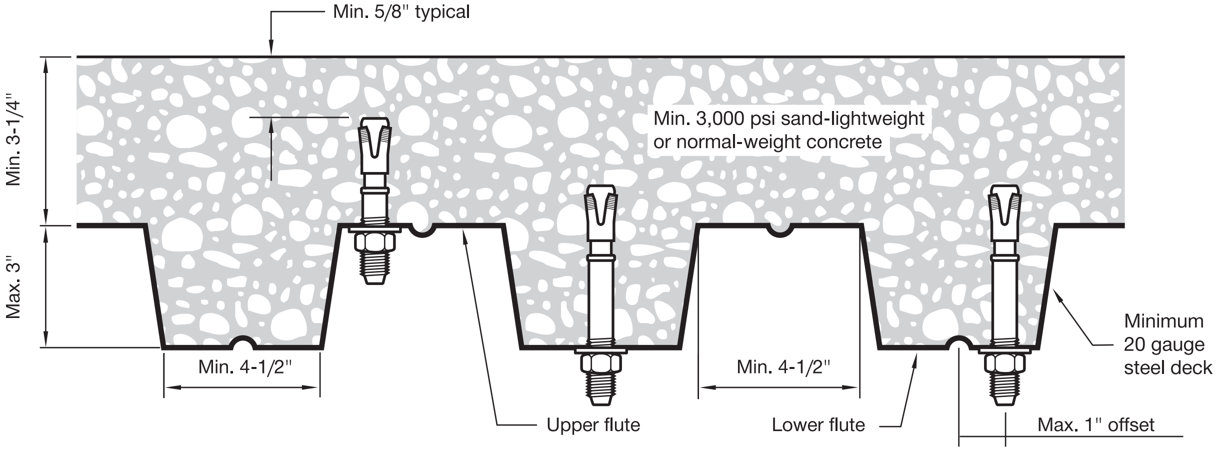


Figure 5 — Installation in concrete over metal deck

DESIGN INFORMATION IN CONCRETE PER CSA A23.3

Limit State Design of anchors is described in the provisions of CSA A23.3 Annex D for post-installed anchors tested and assessed in accordance with ACI 355.2 for mechanical anchors and ACI 355.4 for adhesive anchors. This section contains the Limit State Design tables with unfactored characteristic loads that are based on the published loads in ICC Evaluation Services ESR-2302. These tables are followed by factored resistance tables. The factored resistance tables have characteristic design loads that are prefactored by the applicable reduction factors for a single anchor with no anchor-to-anchor spacing or edge distance adjustments for the convenience of the user of this document. All the figures in the previous ACI 318 Chapter 17 design section are applicable to Limit State Design and the tables will reference these figures.

For a detailed explanation of the tables developed in accordance with CSA A23.3 Annex D, refer to Section 3.1.8. Technical assistance is available by contacting Hilti Canada at (800) 363-4458 or at www.hilti.com.

Table 19 — Steel resistance for Hilti KWIK Bolt 3 stainless steel anchors[1,2]

Nominal anchor diameter in.	Nominal embedment in. (mm)	Tensile N_{sar}[3] lb (kN)	Shear V_{sar}[4] lb (kN)
1/4	1-11/16 (42.9)	1,565 (7.0)	1,070 (4.8)
3/8	2-3/8 (60.3)	4,690 (20.9)	3,175 (14.1)
1/2	2-1/4 (57.2)	8,600 (38.3)	2,675 (11.9)
	3-1/2 (88.9)		4,425 (19.7)
5/8	3-1/2 (88.9)	13,295 (59.1)	5,710 (25.4)
	4-3/8 (111.1)		9,115 (40.5)
3/4	4-1/4 (108.0)	14,690 (65.3)	7,585 (33.7)
	5-1/2 (139.7)		15,010 (66.8)
1	4-5/8 (117.5)	28,770 (128.0)	7,975 (35.5)
	5-7/8 (149.2)		17,430 (77.5)

1 See Section 3.1.8 to convert factored resistance value to ASD value.

2 Hilti KWIK Bolt 3 stainless steel anchors are to be considered ductile steel elements.

3 Tensile $N_{sar} = A_{se,N}\ \phi_s\ f_{uta}\ R$ as noted in CSA A23.3 Annex D.

4 Shear determined by static shear tests with $V_{sar} < A_{se,V}\ \phi_s\ 0.6\ f_{uta}\ R$ as noted in CSA A23.3 Annex D.

Table 20 — Hilti KWIK Bolt 3 stainless steel design information in accordance with CSA A23.3 Annex D[1]

Design parameter	Symbol	Units	Nominal anchor diameter															Ref
			1/4	3/8		1/2				5/8			3/4			1		A23.3-14
Anchor O.D.	d_a	in.	0.25	0.375		0.5				0.625			0.75			1		
		(mm)	(6.4)	(9.5)		(12.7)				(15.9)			(19.1)			(25.4)		
Effective minimum embedment[2]	h_{ef}	in.	1-1/2	2		2		3-1/4		3-1/8	4		3-3/4		5	4	5-1/4	
		(mm)	(38)	(51)		(51)		(83)		(79)	(102)		(95)		(127)	(102)	(133)	
Minimum concrete thickness[2]	h_{min}	in.	4	4	5	4	6	6	8	5	6	8	6	8	8	8	10	
		(mm)	(102)	(102)	(127)	(102)	(152)	(152)	(203)	(127)	(152)	(203)	(152)	(203)	(203)	(203)	(254)	
Critical edge distance	c_{ac}	in.	3	4-3/8	3-7/8	4-7/8	4	6-3/4	5-3/4	7-3/8	9-1/2	7-1/2	10-1/2	9-1/4	9-3/4	10	11	
		(mm)	(76)	(111)	(98)	(124)	(102)	(171)	(146)	(187)	(241)	(191)	(267)	(235)	(248)	(254)	(279)	
Minimum edge distance	c_{min}	in.	1-3/8	2	1-5/8	2-1/2	1.875	1-5/8	1-5/8	3-1/4	2-1/2	2-1/2	3-1/4	3	2-7/8	3-1/2	3	
		(mm)	(35)	(51)	(41)	(64)	(48)	(41)	(41)	(83)	(64)	(64)	(83)	(76)	(73)	(89)	(76)	
	for s >	in.	1-3/4	4	3-3/8	5	4-5/8	4-1/2	4.25	5-5/8	5-1/4	5	7	6-7/8	6-5/8	6-3/4	6-3/4	
		(mm)	(44)	(102)	(86)	(127)	(117)	(114)	(108)	(143)	(133)	(127)	(178)	(175)	(168)	(171)	(171)	
Minimum anchor spacing	s_{min}	in.	1-1/4	2	1-3/4	2-1/2	2-1/4	2	1-7/8	3-1/8	2-1/8	2-1/8	4	3-1/2	3-1/2	5	4-3/4	
		(mm)	(32)	(51)	(44)	(64)	(57)	(2)	(48)	(79)	(54)	(54)	(102)	(89)	(89)	(127)	(121)	
	for c >	in.	1-5/8	3-1/4	2-1/2	2-7/8	2-3/8	2-3/8	2-1/8	3-7/8	3	2-3/4	4-1/8	3-3/4	3-3/4	4-1/4	3-3/4	
		(mm)	(41)	(83)	(64)	(73)	(60)	(60)	(54)	(98)	(76)	(70)	(105)	(95)	(95)	(108)	(95)	
Minimum hole depth in concrete	h_o	in.	2	2-5/8		2-5/8		4		3-7/8	4-3/4		4-1/2		5-3/4	5	6-3/4	
		(mm)	(50.8)	(67)		(67)		(102)		(98)	(121)		(117)		(146)			
Minimum specified yield strength	f_{ya}	psi	84,800	92,000		92,000				92,000			76,000			76,000		
		(N/mm^2)	(585)	(634)		(634)				(634)			(524)			(524)		
Minimum specified ultimate strength	f_{uta}	psi	115,000	115,000		115,000				115,000			90,000			90000		
		(N/mm^2)	(793)	(793)		(793)				(793)			(621)			(621)		
Effective tensile stress area	$A_{se,N}$	in^2	0.02	0.06		0.11				0.17			0.24			0.47		
		(mm^2)	(12.9)	(38.7)		(71.0)				(109.7)			(154.8)			(154.8)		
Steel embedment material resistance factor for reinforcement	ϕ_s	-	0.85															8.4.3
Resistance modification factor for tension, steel failure modes[3]	R	-	0.80															D.5.3
Resistance modification factor for shear, steel failure modes[3]	R	-	0.75															D.5.3
Factored steel resistance in tension	N_{sar}	lb	1,565	4,690		8,600				13,295			14,690			28,770		D.6.1.2
		(kN)	(7.0)	(20.9)		(38.3)				(59.1)			(65.3)			(128.0)		
Factored steel resistance in shear	V_{sar}	lb	1,070	3,175		2,675		4,425		5,710	9,115		7,585		15,010	7,975	17,430	D.7.1.2
		(kN)	(4.8)	(14.1)		(11.9)		(19.7)		(25.4)	(66.8)		(33.7)		(66.8)	(35.5)	(77.5)	
Coeff. for factored concrete breakout resistance, uncracked concrete	$k_{c,uncr}$	-	10															D.6.2.2
Modification factor for anchor resistance, tension, uncracked concrete[4]	$\psi_{c,N}$	-	1.0															D.6.2.6
Anchor category	-	-	2	1														D.5.3 (c)
Concrete material resistance factor	ϕ_c	-	0.65															8.4.2
Resistance modification factor for tension and shear, concrete failure modes, Condition B[5]	R	-	0.85	1.00														D.5.3 (c)
Factored pullout resistance in 20 MPa uncracked concrete[6]	$N_{pr,uncr}$	lb	1,100	2,070		2,315		4,225		4,360	5,485		6,000		7,600	NA	10,905	D.6.3.2
		(kN)	(4.9)	(9.2)		(10.3)		(18.8)		(19.4)	(24.4)		(26.7)		(33.8)		(48.5)	

1 Design information in this table is taken from ICC-ES ESR-2302, dated December 1, 2020 table 4, and converted for use with CSA A23.3 Annex D.

2 See figure 1 of this section.

3 The stainless steel KWIK Bolt 3 is considered a ductile steel element as defined by CSA A23.3 Annex D section D.2.

4 For all design cases, $\psi_{c,N}$ = 1.0. The appropriate coefficient for breakout resistance for uncracked concrete ($k_{c,uncr}$) must be used.

5 For use with the load combinations of CSA A23.3 chapter 8. Condition B applies where supplementary reinforcement in conformance with CSA A23.3 section D.5.3 is not provided, or where pullout or pryout strength governs. For cases where the presence of supplementary reinforcement can be verified, the resistance modification factors associated with Condition A may be used.

6 For all design cases, $\psi_{c,P}$ = 1.0. NA (not applicable) denotes that this value does not control for design. See section 4.1.4 of ESR-2302 for additional information.

Table 21 — Hilti KWIK Bolt 3 stainless steel factored resistance with concrete/pullout failure in uncracked concrete[1,2,3,4,5]

Nominal anchor diameter in.	Effective embed. in. (mm)	Nominal embed. in. (mm)	Tension - N_r				Shear - V_r			
			f'_c = 20 MPa (2,900psi) lb (kN)	f'_c = 25 MPa (3,625 psi) lb (kN)	f'_c = 30 MPa (4,350 psi) lb (kN)	f'_c = 40 MPa (5,800 psi) lb (kN)	f'_c = 20 MPa (2,900 psi) lb (kN)	f'_c = 25 MPa (3,625 psi) lb (kN)	f'_c = 30 MPa (4,350 psi) lb (kN)	f'_c = 40 MPa (5,800 psi) lb (kN)
1/4	1 1/2 (38)	1-11/16 43	930 (4.1)	1,040 (4.6)	1,140 (5.1)	1,315 (5.8)	1,300 (5.8)	1,455 (6.5)	1,595 (7.1)	1,840 (8.2)
3/8	2 (51)	2-5/16 (59)	2,080 (9.2)	2,325 (10.3)	2,545 (11.3)	2,940 (13.1)	2,380 (10.6)	2,660 (11.8)	2,915 (13.0)	3,365 (15.0)
1/2	2 (51)	2 3/8 (60)	2,315 (10.3)	2,585 (11.5)	2,835 (12.6)	3,275 (14.6)	2,380 (10.6)	2,660 (11.8)	2,915 (13.0)	3,365 (15.0)
	3-1/4 (83)	3 5/8 (92)	4,220 (18.8)	4,715 (21.0)	5,165 (23.0)	5,965 (26.5)	9,885 (44.0)	11,050 (49.2)	12,105 (53.8)	13,975 (62.2)
5/8	3-1/8 (79)	3-9/16 (90)	4,360 (19.4)	4,875 (21.7)	5,340 (23.8)	6,165 (27.4)	9,175 (40.8)	10,260 (45.6)	11,240 (50.0)	12,980 (57.7)
	4 (102)	4-7/16 (113)	5,480 (24.4)	6,125 (27.2)	6,710 (29.8)	7,750 (34.5)	13,465 (59.9)	15,055 (67.0)	16,490 (73.4)	19,040 (84.7)
3/4	3-3/4 (95)	4-5/16 (110)	6,000 (26.7)	6,705 (29.8)	7,345 (32.7)	8,480 (37.7)	12,100 (53.8)	13,530 (60.2)	14,820 (65.9)	17,115 (76.1)
	4-3/4 (121)	5-9/16 (141)	7,590 (33.8)	8,485 (37.7)	9,295 (41.3)	10,730 (47.7)	17,395 (77.4)	19,450 (86.5)	21,305 (94.8)	24,600 (109.4)
1	4 (102)	4-5/16 (110)	6,730 (29.9)	7,525 (33.5)	8,245 (36.7)	9,520 (42.3)	13,465 (59.9)	15,055 (67.0)	16,490 (73.4)	19,040 (84.7)
	5 3/4 (146)	5-9/16 (141)	10,895 (48.5)	12,180 (54.2)	13,340 (59.3)	15,405 (68.5)	23,055 (102.6)	25,780 (114.7)	28,240 (125.6)	32,610 (145.0)

1 See section 3.1.8 to convert factored resistance value to ASD value.

2 Linear interpolation between embedment depths and concrete compressive strengths is not permitted.

3 Apply spacing, edge distance, and concrete thickness factors in tables 14 to 19 as necessary. Compare to the steel values in table 31. The lesser of the values is to be used for the design.

4 Tablular values are for normal-weight concrete only. For lightweight concrete multiply design strength by λ_a as follows: for sand-lightweight, λ_a = 0.68; for all-lightweight, λ_a = 0.60

5 Tabular values are for static loads only. Seismic design is not permitted for uncracked concrete.

Table 22 — Steel resistance for Hilti KWIK Bolt 3 hot-dip galvanized carbon steel anchors[1,2]

Nominal anchor diameter in.	Nominal embedment in. (mm)	Tensile N_{sar} [3] lb (kN)	Shear V_{sar} [4] lb (kN)
1/2	2-1/4 (57)	7,930 (35.3)	2,870 (12.8)
	3-1/2 (89)		3,740 (16.6)
5/8	3-1/2 (89)	12,255 (54.5)	7,415 (33.0)
	4-3/8 (111)		
3/4	4-1/4 (108)	17,300 (77.0)	10,840 (48.2)
	5-1/2 (140)		

1 See section 3.1.8 to convert design strength value to ASD value.

2 KWIK Bolt 3 hot-dip galvanized carbon steel anchors are to be considered ductile steel elements.

3 Tensile $N_{sar} = A_{se,N}\ \phi_s\ f_{uta}$ R as noted in ACI 318 Chapter 17.

4 Shear values determined by static shear tests with $V_{sar} < A_{se,V}\ \phi_s\ 0.6\ f_{uta}$ R as noted in ACI 318 Chapter 17.

Table 23 — Hilti KWIK Bolt 3 hot-dip galvanized carbon steel design information in accordance with CSA A23.3 Annex D[1]

Design parameter	Symbol	Units											Ref A23.3-14
			1/2				5/8			3/4			
Anchor O.D.	d_a	in.	0.5				0.625			0.75			
		(mm)	(12.7)				(15.9)			(19.1)			
Effective minimum embedment[2]	h_{ef}	in.	2		3-1/4		3-1/8	4		3-3/4		4-3/4	
		(mm)	(51)		(83)		(79)	(102)		(95)		(121)	
Minimum concrete thickness	h_{min}	in.	4	6	6	8	5	6	8	6	8	8	
		(mm)	(102)	(152)	(152)	(203)	(127)	(152)	(203)	(152)	(203)	(203)	
Critical edge distance	c_{ac}	in.	4-7/8	3-5/8	7-1/2	5.75	8	9-1/2	8	9-3/4	7-1/2	9-1/2	
		(mm)	(124)	(92)	(191)	(146)	(194)	(241)	(197)	(248)	(191)	(241)	
Minimum edge distance	c_{min}	in.	2-7/8		2-1/8		3-1/4	2-3/8		4-1/4		4	
		(mm)	(73)		(54)		(83)	(60)		(108)		(102)	
	for s >	in.	5-3/4		5-1/4		5-1/2	5-1/2		10		8-1/2	
		(mm)	(146)		(133)		(140)	(140)		(254)		(216)	
Minimum anchor spacing	s_{min}	in.	2-7/8		2		2-3/4	2-3/8		5		4	
		(mm)	(73)		(51)		(70)	(60)		(127)		(102)	
	for c >	in.	4-1/2		3-1/4		4-1/8	4-1/4		9-1/2		7	
		(mm)	(114)		(83)		(105)	(108)		(241)		(178)	
Minimum hole depth in concrete	h_o	in.	2-5/8		4		3-3/4	4-3/4		4-1/2		5-3/4	
		(mm)	(67)		(102)		(98)	(121)		(117)		(146)	
Minimum specified yield strength	f_{ya}	psi	92,000				92,000			76,125			
		(N/mm²)	(634)				(634)			(525)			
Minimum specified ultimate strength	f_{uta}	psi	115,000				115,000			101,500			
		(N/mm²)	(793)				(793)			(700)			
Effective tensile stress area	$A_{se,N}$	in²	0.101				0.162			0.237			
		(mm²)	(65.0)				(104.6)			(152.8)			
Steel embedment material resistance factor for reinforcement	ϕ_s	-	0.85										8.4.3
Resistance modification factor for tension, steel failure modes[4]	R	-	0.80										D.5.3
Resistance modification factor for shear, steel failure modes[4]	R	-	0.75										D.5.3
Factored steel resistance in tension	N_{sar}	lb	7,930				12,255			17,300			D.6.1.2
		(kN)	(35.3)				(54.5)			(77.0)			
Factored steel resistance in shear	V_{sar}	lb	2,870		3,740		7,415			10,840			D.7.1.2
		(kN)	(12.8)		(16.6)		(33.0)			(48.2)			
Coefficient for factored concrete breakout resistance, uncracked concrete	$k_{c,uncr}$	-	10										D.6.2.2
Modification factor for anchor resistance, tension, uncracked concrete[4]	$\psi_{c,N}$	-	1.00										D.6.2.6
Anchor category	-	-	1										D.5.3 (c)
Concrete material resistance factor	ϕ_c	-	0.65										8.4.2
Resistance modification factor for tension and shear, concrete failure modes, Condition B[5]	R	-	1.00										D.5.3 (c)
Factored pullout resistance in 20 MPa uncracked concrete[6]	$N_{pr,uncr}$	lb	N/A		4,585		4,540	6,315		NA		7,125	D.6.3.2
		(kN)			(20.4)		(20.2)	(28.1)				(31.7)	

1 Design information in this table is taken from ICC-ES ESR-2302, dated December 1, 2020 table 4, and converted for use with CSA A23.3 Annex D.
2 See figure 1 of this section.
3 The hot-dip galvanized carbon steel KWIK Bolt 3 is considered a ductile steel element as defined by CSA A23.3 Annex D section D.2.
4 For all design cases, $\psi_{c,N}$ = 1.0. The appropriate coefficient for breakout resistance for uncracked concrete ($k_{c,uncr}$) must be used.
5 For use with the load combinations of CSA A23.3 chapter 8. Condition B applies where supplementary reinforcement in conformance with CSA A23.3 section D.5.3 is not provided, or where pullout or pryout strength governs. For cases where the presence of supplementary reinforcement can be verified, the resistance modification factors associated with Condition A may be used.
6 For all design cases, $\psi_{c,P}$ = 1.0. NA (not applicable) denotes that this value does not control for design. See section 4.1.4 of ESR-2302 for additional information.

Table 24 — Hilti KWIK Bolt 3 hot-dip galvanized carbon steel factored resistance with concrete / pullout failure in uncracked concrete[1,2,3,4,5]

Nominal anchor diameter in.	Effective embed. in. (mm)	Nominal embed. in. (mm)	Tension - N_r				Shear - V_r			
			f'_c = 20 MPa (2,900psi) lb (kN)	f'_c = 25 MPa (3,625 psi) lb (kN)	f'_c = 30 MPa (4,350 psi) lb (kN)	f'_c = 40 MPa (5,800 psi) lb (kN)	f'_c = 20 MPa (2,900 psi) lb (kN)	f'_c = 25 MPa (3,625 psi) lb (kN)	f'_c = 30 MPa (4,350 psi) lb (kN)	f'_c = 40 MPa (5,800 psi) lb (kN)
1/2	2 (51)	2 3/8 (60)	2,380 (10.6)	2,660 (11.8)	2,915 (13.0)	3,365 (15.0)	2,380 (10.6)	2,660 (11.8)	2,915 (13.0)	3,365 (15.0)
	3-1/4 (83)	3 5/8 (92)	4,580 (20.4)	5,120 (22.8)	5,610 (25.0)	6,480 (28.8)	9,885 (44.0)	11,050 (49.2)	12,105 (53.8)	13,975 (62.2)
5/8	3-1/8 (79)	3-9/16 (90)	4,535 (20.2)	5,070 (22.5)	5,555 (24.7)	6,410 (28.5)	9,175 (40.8)	10,260 (45.6)	11,240 (50.0)	12,980 (57.7)
	4 (102)	4-7/16 (113)	6,315 (28.1)	7,060 (31.4)	7,730 (34.4)	8,930 (39.7)	13,465 (59.9)	15,055 (67.0)	16,490 (73.4)	19,040 (84.7)
3/4	3-3/4 (95)	4-5/16 (110)	6,050 (26.9)	6,765 (30.1)	7,410 (33.0)	8,555 (38.1)	12,100 (53.8)	13,530 (60.2)	14,820 (65.9)	17,115 (76.1)
	4-3/4 (121)	5-9/16 (141)	7,130 (31.7)	7,975 (35.5)	8,735 (38.9)	10,085 (44.9)	17,395 (77.4)	19,450 (86.5)	21,305 (94.8)	24,600 (109.4)

1 See section 3.1.8 to convert factored resistance value to ASD value.

2 Linear interpolation between embedment depths and concrete compressive strengths is not permitted.

3 Apply spacing, edge distance, and concrete thickness factors in tables 23 to 25 as necessary. Compare to the steel values in table 34. The lesser of the values is to be used for the design.

4 Tablular values are for normal-weight concrete only. For lightweight concrete multiply design strength by λ_a as follows: for sand-lightweight, λ_a = 0.68; for all-lightweight, λ_a = 0.60

5 Tabular values are for static loads only. Seismic design is not permitted for uncracked concrete.

Table 25 — Hilti KWIK Bolt 3 stainless steel factored resistance in the soffit of uncracked lightweight concrete over metal deck[1,2,3,4,5,6,7]

Nominal anchor diameter in.	Effective embed. in. (mm)	Nominal embed. in. (mm)	Loads according to figure 5		
			Tension - N_r		Shear - V_r
			f'_c = 20 MPa (2,900 psi) lb (kN)	f'_c = 30 MPa (4,350 psi) lb (kN)	f'_c ≥ 20 MPa (2,900 psi) lb (kN)
1/4	1-1/2 (38)	1-11/16 (43)	980 (4.4)	1,200 (5.3)	1,290 (5.7)
3/8	2 (51)	2-5/16 (59)	1,650 (7.3)	2,020 (9.0)	1,645 (7.3)
1/2	2 (51)	2-3/8 (60)	1,245 (5.5)	1,520 (6.8)	1,110 (4.9)
	3-1/4 (83)	3-5/8 (92)			
5/8	3-1/8 (79)	3-9/16 (90)	2,830 (12.6)	3,465 (15.4)	3,625 (16.1)
	4 (102)	4-7/16 (113)			

1 See section 3.1.8 to convert design strength value to ASD value.

2 Linear interpolation between embedment depths and concrete compressive strengths is not permitted.

3 Tabular value is for one anchor per flute. Minimum spacing along the length of the flute is 3 x h_{ef} (effective embedment).

4 Tabular value is for lightweight concrete and no additional reduction factor is needed.

5 No additional reduction factors for spacing or edge distance need to be applied.

6 Comparison of the tabular values to the steel strength is not necessary. Tabular values control.

7 Tabular values are for static loads only. Seismic design is not permitted for uncracked concrete.

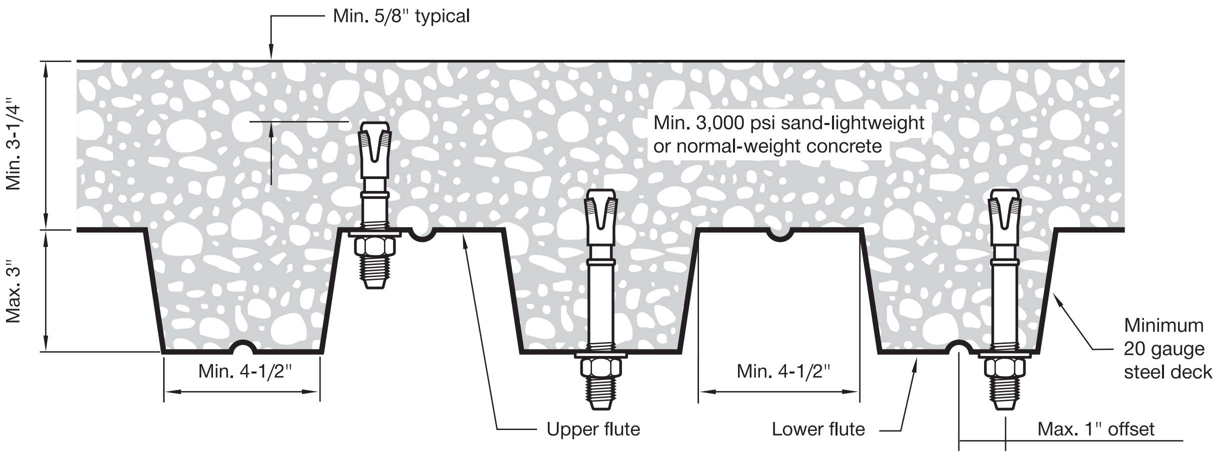


Figure 5 — Installation in concrete over metal deck

DESIGN INFORMATION FOR COUNTERSUNK KWIK BOLT 3

Table 26 — Countersunk Hilti KWIK Bolt 3 allowable loads in normal-weight concrete[1]

Anchor Material	Nominal anchor diameter in.	Embedment depth in. (mm)		f'_c = 3000 psi (20.7 MPa)			
				Tension lb (kN)		Shear[2] lb (kN)	
Carbon Steel	1/4	1-1/8	(29)	365	(1.6)	350	(1.6)
	3/8	1-5/8	(41)	810	(3.6)	750	(3.3)
Stainless Steel	1/4	1-1/8	(29)	320	(1.4)	500	(2.2
	3/8	1-5/8	(41)	670	(3.0)	1330	(5.9)

1 Allowable loads based on using a safety factor of 4.0.

2 Shear values acting thru threads of anchor bolt. If acting through the empty shell, reduce loads by 70%.

INSTALLATION INSTRUCTIONS

Installation Instructions For Use (IFU) are included with each product package. They can also be viewed or downloaded online at www.hilti.com. Because of the possibility of changes, always verify that downloaded IFU are current when used. Proper installation is critical to achieve full performance. Training is available on request. Contact Hilti Technical Services for applications and conditions not addressed in the IFU.

ORDERING INFORMATION

KWIK Bolt 3 anchor product line

Size	Length (ℓ)		Thread length (ℓ_{th})		ID stamp	Box	304 SS	316 SS	HDG
	in.	(mm)	in.	(mm)					
1/4 x 1-3/4	1-3/4	(44)	3/4	(18)	A	100	●		
1/4 x 2-1/4	2-1/4	(57)	7/8	(22)	B		●	●	
1/4 x 3-1/4	3-1/4	(83)	2	(51)	D		●		
			7/8	(22)				●	
1/4 x 4-1/2	4-1/2	(114)	2-7/8	(75)	G		●		
3/8 x 2-1/4	2-1/4	(57)	7/8	(22)	B	50	●		
3/8 x 3	3	(76)	1-1/4	(32)	D			●	
			1-1/2	(40)			●		
3/8 x 3-3/4	3-3/4	(95)	1-1/4	(32)	E			●	
			2-1/4	(59)			●		
3/8 x 5	5	(127)	3-1/2	(91)	H		●		
3/8 x 7	7	(178)	5-1/2	(142)	L		●		
1/2 x 2-3/4	2-3/4	(70)	1-1/4	(33)	C	25	●		
1/2 x 3-3/4	3-3/4	(95)	1-5/16	(35)	E			●	
			2-3/16	(56)			●		●
1/2 x 4-1/2	4-1/2	(114)	1-5/16	(35)	G			●	
			2-7/8	(75)			●		●
1/2 x 5-1/2	5-1/2	(140)	1-5/16	(35)	I			●	
			3-3/4	(96)			●		●
1/2 x 7	7	(178)	4-3/4	(121)	L		●		●
5/8 x 3-3/4	3-3/4	(95)	1-1/2	(41)	E	15	●	●	
5/8 x 4-3/4	4-3/4	(121)	1-1/2	(41)	G			●	
			2-3/4	(70)			●		●
5/8 x 6	6	(152)	1-1/2	(41)	J			●	
			4	(102)			●		●
5/8 x 7	7	(178)	4-3/4	(121)					
5/8 x 8-1/2	8-1/2	(216)	6-1/2	(166)	O		●		
5/8 x 10	10	(254)	7	(180)	R		●		
3/4 x 4-3/4	4-3/4	(121)	1-1/2	(41)	G	20	●	●	
			2-7/16	(62)		10			●
						20	●		
3/4 x 5-1/2	5-1/2	(140)	1-1/2	(41)	I	20	●		
			3-7/16	(85)		10			●
						20	●		
3/4 x 7	7	(178)	4-5/8	(119)	L	10			
			4-7/8	(124)			●		
3/4 x 8	8	(203)	5-3/4	(146)	N		●		●
3/4 x 10	10	(254)	5-7/8	(152)	R		●	●	
3/4 x 12	12	(305)	5-7/8	(152)	T		●		
1 x 6	6	(152)	2-1/4	(57)	J	5	●	●	
1 x 9	9	(114)	2-1/4	(57)	P		●		
1 x 12	12	(114)	6	(152)	T		●		

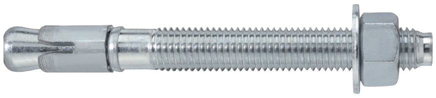
Countersunk KWIK Bolt 3 anchor product line

Size	Length in.	Length (mm)	Box	Carbon steel	304 SS
C1/4 x 2	2	(51)	100	•	
C1/4 x 3	3	(76)	100	•	•
C1/4 x 5	5	(127)	100	•	
C3/8 x 2-1/4	2-1/4	(57)	100	•	
C3/8 x 3	3	(76)	100	•	
C3/8 x 4	4	(102)	50	•	•
C3/8 x 5	5	(127)	50	•	

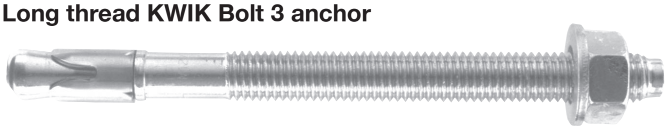
Rod Coupling KWIK Bolt 3 anchor product line

Size	Length in.	Length (mm)	Thread length in.	Thread length (mm)	ID stamp	Box quantity
3/8 x 2-1/4	2-1/4	(57)	7/8	(22)	B	100

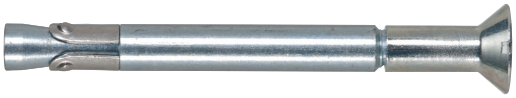
KWIK Bolt 3 anchor



Long thread KWIK Bolt 3 anchor



Countersunk KWIK Bolt 3 anchor



Rod coupling KWIK Bolt 3 anchor 3/8 x 2 1/4

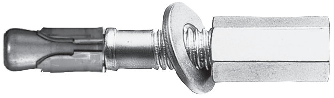


Table 43 — KWIK Bolt 3 length identification system

Length ID marking on bolt head		A	B	C	D	E	F	G	H	I	J	K	L	M	N	O	P	Q	R	S	T	U	V	W
Length of anchor, ℓ_{anch} in.	From	1 ½	2	2 ½	3	3 ½	4	4 ½	5	5 ½	6	6 ½	7	7 ½	8	8 ½	9	9 ½	10	11	12	13	14	15
	Up to but not including	2	2 ½	3	3 ½	4	4 ½	5	5 ½	6	6 ½	7	7 ½	8	8 ½	9	9 ½	10	11	12	13	14	15	16